16 Latitude: 38°07′05.00″

Structure Description: 339.9 Foot - 3 Span Steel continuous Stringer/Multi-beam or Girder

2 District: 09 **3 County:** Bath

7 Facility Carried I-64-10 NC

6A Feature Intersected: SLATE CREEK

9 Location: WBL 2.8MI E-MONTGOMERY CL

NBI CONDITION RATINGS

58	Deck:	7	61 Channel:	7
59	Superstructure:	4	62 Culvert:	Ν
60	Substructure:	6	Sufficiency Rating:	91

DE	SIGN
Substandard:	No
Fracture Critical:	No
43A Main Span Material:	(4) Steel Continuous
43B Main Span Design:	(02) Stringer / Girder
45 Number of Spans Main:	3
44A Approach Span Material:	Not Applicable
44B Approach Span Design:	Not Applicable
46 Number of Approach Spar	15: 0
107 Deck Type:	(1) Concrete-Cast-in-Place
108A Wearing Surface:	(2) Integral Concrete
108B Membrane:	(0) None
108C Deck Protection:	(0) None
Overlay Y/N:	No
Overlay Type:	None
Overlay Thickness:	0.000 in
Overlay Date:	

	APPRAISAL						
36A	Bridge Railings:	(1) Meets Standards					
36B	Transitions	(1) Meets Standards					
36C	Approach Guardrail:	(1) Meets Standards					
36D	Approach Guardrail Ends:	(1) Meets Standards					
71	Waterway Adequacy:	(8) Equal Desirable					
72	Approach Alignment:	(8) Equal Desirable Crit					
113	Scour Critical:	(8) Stable above footing					
Reco	mmended Scour Critical:	(8) Stable above footing					

LOAD RATINGS

63	Operating Type:	(1) Load Factor (LF)
64	Operating Rating:	63.1 tons
65	Inventory Type:	(1) Load Factor (LF)
66	Inventory Rating:	38.0 tons
Truck	Capacity Type I:	53 tons
Truck	Capacity Type II:	54 tons
Truck	Capacity Type III:	57 tons
Truck	Capacity Type IV:	65 tons

7 Longitude: 83°47′57.00″
Longitude. 00 47 07.00

Milepoint: 118.380

NBI	Х
Element	Х
Fracture Critical	
Underwater	
Special	

	GEOMETRIC DATA						
48	Max Length Span:	140.092 ft					
49	Structure Length:	339.895 ft					
32	Approach Roadway:	37.073 ft					
33	Median:	(0) No Median					
34	Skew:	20°					
35	Flare:	No Flare					
50A	Curb/Sidewalk Width L:	0.000 ft					
50B	Curb/Sidewalk Width R:	0.000 ft					
47	Horiz. Clearance:	32.999 ft					
51	Width Curb to Curb:	32.999 ft					
52	Width Out to Out:	36.001 ft					

	ADMINISTRATIVE						
27	Year Built:	1967					
106	Year Reconstructed:	-4					
42A	Type of Service On:	(1) Highway					
42B	Type of Service Under:	(5) Waterway					
37	Historical Significance:	(5) Not Eligible					
21	Custodian:	(01) State Hwy Agency					
22	Owner:	(01) State Hwy Agency					
101	Parallel Structure:	(L) Left Of II Structure					

	CLEARA	NCES
10	Vert. Clearance:	99.999 ft
53	Min. Vert. Clearance Over:	99.999 ft
54A	Vert. Under Reference:	(N) Feature not hwy or RR
54B	Min. Vert. Underclearance:	0.000 ft
55A	Lateral Under Reference:	(N) Feature not hwy or RR
55B	Min. Lat. Underclearance R:	0.000 ft
56	Min. Lat. Underclearance L:	0.000 ft

POSTINGS							
41 Posting Status: (A) Open, No Restriction							
Signs Posted Cardinal:	No						
Signs Posted Non-Cardinal:	No						
Field Postings Gross:	tons						
Field Postings Type I:	tons						
Field Postings Type II:	tons						
Field Postings Type III:	tons						
Field Postings Type IV:	tons						

12: Re Concrete Deck									
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
SQ.FT	12,236.44	11,786.44	96%	450	4%	0	0%	0	0%
	ring surface has m up. See photos.	inor sized transve	erse and lo	ngitudinal cracks t	throughout.	A few of these cr	acks in spa	an 2 are beginning	

107: Steel Opn Girder/Beam										
Units	Units Total Qty Qty. St. 1 % in 1 Qty. St. 2 % in 2 Qty. St. 3 % in 3 Qty. St. 4 % in									
FT	1,360	1,017	75%	341	25%	0	0%	2	0%	
during a behind th downwar opens up bearing. edge of t that was location a noticeabl bearing.	routine inspection ne rocker bearing of and appears to of and partially closs This crack extend he bottom flange. discovered in 201 along the web/bot ly bends downwar The north girder a	approximately 1.5 stop at the bottom ses. A NEW CRAC Is across the full w This crack becom 1. These two crac	web thickne in. off the l of the ver X was disc idth and de es hairline ks are ver il stiffener ing. The bo moderate p	ess at this location bottom flange. The tical stiffener weld covered during this epth of the bottom in width at the we y close to each oth weld. The bearing ottom flange of gird bitting in the bottor	a is thin. On e crack extend and bottom s inspection flange. The b/bottom flaner and mo below this der 2 from to m of the we	te end of this crac ends inward (towa n flange. When th n in this same gird e crack is almost ange weld and rui re than likely they girder is froze an the south also ber	k is at the ward the bear e bridge is ler just in fr 1/8 in wide ns back tow initiated fro d the end conds down s	very end of the wel ring) and then turns loaded the crack ont the rocker at the southern vard the web crack om the same of the bottom flange lightly behind the	S	

515: Ste	el Protective Co	ating							
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	8,839.2	7,741.92	88%	91.44	1%	91.44	1%	914.4	10%

The paint area for these girders was supplied by Tom Mathews from Central Office. The protective coatings have large widespread failures where the paint has just "popped" off exposing the steel throughout these girders. Loose paint is peeling/flaking around these locations. The exterior girders have bubbling paint along their exterior bottom flanges. The ends of the beams at the abutments also have some flaking/ peeling paint with minor to moderate rust/corrosion. The girders need a new protective coating. See photos.

1000: Corrosion												
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4			
FT	1	1	100%	0	0%	0	0%	0	0%			
See element 107 for details.												

	1010: Cracking												
Units Total Qty Qty. St. 1 % in 1 Qty. St. 2 % in 2 Qty. St. 3 % in 3	Qty. St. 4	% in 4											
FT 1 1 100% 0 0% 0 0%	0	0%											

205: Re	205: Re Conc Column												
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4				
EACH	4	4	100%	0	0%	0	0%	0	0%				
The bridg photos.	e consists of two	piers with two squ	uare colum	ns in each. These	concrete c	olumns are in goo	od conditior	n at this time. See					

215: Re	Conc Abutment								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	132	92	70%	40	30%	0	0%	0	0%

Abutment 1 has some light to minot cracks in the backwall. A moderate sized spall with shallow exposed steel is present in abutment 1s backwall between beams 3 and 4 from the south. The breast wall of abutment 4 has minor sized horizontal cracking for ~ 75% of its length. Abutment 4s backwall has some areas of minor cracking. See photos.

234: Re Conc Pier Cap												
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4			
FT	76	76	100%	0	0%	0	0%	0	0%			
Both con	crete pier caps ar	e in good condition	n.									

300: Str	ip Seal Exp Joint	t							
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
FT	36	0	0%	36	100%	0	0%	0	0%
	•					d debris. 1.5 in. wa oximately 70 degro		ed at the roadway hotos.	

36 34 94% 2 6% 0 0% 0 0% ge has a compression seal joint at abutment 4. It is partially full of dirt and debris. The gland has a few small tears, some	FT363494%26%00%0This bridge has a compression seal joint at abutment 4. It is partially full of dirt and debris. The gland has a few small tears, some
ge has a compression seal joint at abutment 4. It is partially full of dirt and debris. The gland has a few small tears, some	This bridge has a compression seal joint at abutment 4. It is partially full of dirt and debris. The gland has a few small tears, some

311: Mo	veable Bearing								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
EACH	12	0	0%	8	67%	0	0%	4	33%

Moveable bearings are present at both abutments and pier 2. The bearings at abutment 1 typically have flaking paint with moderate corrosion in the lower portions of the rockers. Their masonry plates are rusted with moderate to heavy corrosion. Bearing 3 from the south is frozen and is missing the north anchor bolt. The exterior bearings at abutment 1 are tilted back toward the backwall more than the interior bearings. Plumb measurements were taken during this inspection. These measurements were taken on the tapered faces of the rockers so the actual degree of tilt is less. The measurements at abutment 1 are as follows: Bearing 1 from the south is tilted back toward the backwall ~ 18 degrees, bearing 2 ~ 10 degrees, bearing 3 ~15 degrees, and the north bearing ~ 21 degrees. The measurements at abutment 4 are: Bearing 1 from the south is tilted back toward the backwall ~ 10 degrees, bearing 2 ~ 3 degrees, bearing 3 ~12 degrees, and the north bearing ~ 10 degrees. The moveable bearings over the pier appear to be in good condition. The bearings at abutment 4 typically have flaking paint with moderate to heavy corrosion in the lower portions of the rockers and masonry plates. The lower rockers of bearings 1, 3, and 4 from the south extend out beyond the front edge of the masonry plate approximately 1 inch. The front edge of the masonry plate of bearing 2 from the south is slightly out in front of its rocker. This bearing is almost vertical while bearings 1, 3, and 4 are tilted to the rear. This bearing appears to be froze. Both anchor bolts are missing from bearing 2. Under live load this bearing appears to hover or float back and forth slightly, instead of rocking back and forth. The bearings should be cleaned and coated. See photos.

Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
EACH	6.69	6.69	100%	0	0%	0	0%	0	0%

313: Fixed Bearing												
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4			
EACH	4	4	100%	0	0%	0	0%	0	0%			
The fixed	l bearings over pie	er 3 appear to be i	n good cor	ndition. See photos	5.							

515: Ste	el Protective Co	ating							
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
EACH	2.23	2.23	100%	0	0%	0	0%	0	0%

The protective coatings on the fixed bearings over pier 3 appear to be in good condition.

738 718 97% 20 3% 0 0% 0 0% bridge bas constate barriers along both sides. They have the typical light to minor sized vertical cracks scattered throughout	FT73871897%203%00%0This bridge has concrete barriers along both sides. They have the typical light to minor sized vertical cracks scattered throughout.
bridge has concrete barriers along both sides. They have the typical light to minor sized vertical cracks scattered throughout	This bridge has concrete barriers along both sides. They have the typical light to minor sized vertical cracks scattered throughout.

850: 2nd Elem

Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	0	0%	1	100%	0	0%
Minor rus	sting and corrosion	n is present within		ne steel diaphragm	n cross bra	cings. More mode	rate rusting	and corrosion are	3
present w	vithin some of the	diaphragms at the	e abutment	s. See photos.		Ū			-

857: Em	bankment Erosi	on							
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	1	100%	0	0%	0	0%
Some mo	oderate erosion is	present in the em	bankment	in front of abutme	nt 4.				

859: Ve	getation								
Units	Total Qty	Qty. St. 1	% in 1	Qty. St. 2	% in 2	Qty. St. 3	% in 3	Qty. St. 4	% in 4
(EA)	1	0	0%	0	0%	0	0%	1	100%
Vegetatio	on growing on and	l around the bridge	e needs to	be cut and spraye	d. See pho	otos.			
<u> </u>									

STRUCTURE NOTES

-186.6

INSPECTION NOTES

This is a routine 24 month walk over inspection. The notes and NBI ratings reflect what can only be reasonably observed during this type of inspection. Binoculars were utilized for a better visual observation. A new crack was discovered during this inspection in girder 3 from the south at abutment 1 just in front the rocker bearing. This crack extends across the full width and depth of the bottom flange. This structure is being placed on a 3 month inspection cycle to monitor this location. Inspected by A.Greiner.

	WORK			
Action:	1011 - Bearings-Rehabiliatation			
Some of the bearings at the abutments need repairs and they all need to cleaned and coated. Generated by user "agreiner" on 5/29/2015				

Action:	1079 - Superstructure-Repair Steel
Girder 3	B from the south at abutment 1 needs to be repaired. Generated by user "agreiner" on 5/29/2015

Action: 7 - Paint Bridge

The Girders need to be painted. Generated by user "agreiner" on 5/29/2015



Typical view of the wearing surface.



View of a 1.5" measurement taken at the expansion joint at abutment 1 at the roadway centerline.



View of the expansion joint at abutment 1. It is partially full of dirt and debris.



View of light transverse cracking in the right lane in span 2.



View of transverse cracking that is beginning to open up in the right lane near midspan in span 2.



View of a small tare in the seal at the center of the left lane at abutment 4.



View of the transverse joint at abutment 4. It is partially full of dirt and debris.



View from the approach to abutment 4. This section of the interstate was being paved during this inspection.



View of the stamps on the north barrier at abutment 4.



View of light cracking in abutment 4's breast wall below bearing 3 from the south.



View of the north bearing at abutment 4. It has flaking paint and moderate corrosion of the masonry plate.



View of moderate rusting and corrosion at the end of the bottom flange of the north beam at abutment 4.



View of flaking paint along the north face of beam 3's web.



View of bearings 1 and 2 from the south at abutment 4. Bearing 1 is rocked back toward the backwall and bearing 2 is almost straight.



View of the rocker of bearing 3 extending approximately 1" beyond the front edge of the masonry plate.



View of bearings 3 and 4 from the south. They are rocked back toward the backwall.



View of the rocker of bearing 2 from the south. The rocker is slightly behind the front edge of the masonry plate.



View of some moderate rusting and corrosion of the steel diaphragm between beams 1 and 2 from the south at abutment 4.



View of moderate rusting and corrosion at the end of beam 2 from the south at abutment 4.



View of vegetation growth at the south end of span 3.



View of span 3 and pier 3.



View from the north near abutment 1.



View of the north bearing at abutment 1.



View of moderate pitting in the lower web of the north beam behind the bearing at abutment 1.



View of a moderate sized shallow spall with exposed steel in the backwall of abutment 1 between beams 3 and 4 from the south.



View of the previously noted crack in the web of beam 3 from the south at abutment 1. This crack is at the end of the web and stops at the bottom of the vertical stiffener.



View of bearing 3 at abutment 1. Notice the north anchor bolt is missing.



View of the previously noted crack in the web of beam 3 from the south at abutment 1. This crack is at the end of the web and stops at the bottom of the vertical stiffener.



View of the previously noted crack in the web of beam 3 from the south at abutment 1. This crack is at the end of the web and stops at the bottom of the vertical stiffener.



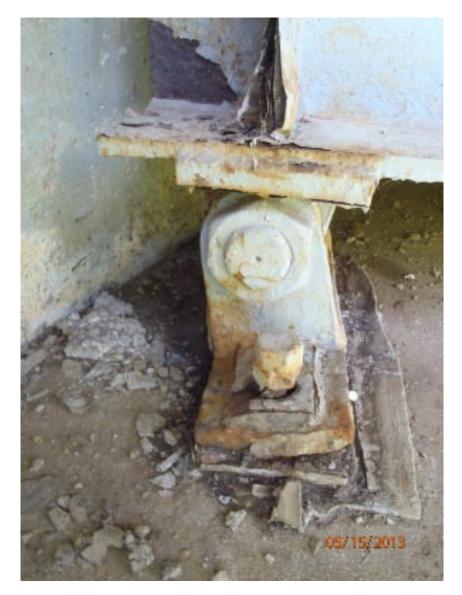
View of span 1 and pier 2.



View of beam 3 from the south at abutment 1. Notice the bottom flange bending downward toward the backwall.



View of span 2.



View of the south bearing at abutment 1. Notice the pack rust at the bottom of the vertical stiffener.